

APPROACH ELEVATION

ELEVATION

DEPARTURE ELEVATION

ALL EXPOSED PAINTED SURFACES SHALL BE COATED WITH MATTHEWS *282-208SP VOC GLOSS CLEAR, WITH MINIMUM 2 MILS DRY FILM THICKNESS (DFT) PER MATTHEWS APPLICATION SPECIFICATIONS.

CANOPY ELECTRICAL REQUIREMENTS (NOT INCLUDING ATM MACHINE, SECURITY CAMERA & RELATED APPLIANCES):

LEDS: (41) 7100°K WHITE SLOAN PRISM *701269-7WSJI-MB (12) BLUE SLOAN PRISM *701269-BLSJI-MB DOWNLIGHTS: (2) E-CONOLIGHT 5000K WHITE LED LIGHT FIXTURE *C-CP-B-SQ-4L-50K-WH © 030A

LED POWER SUPPLY: (2) SLOAN *701507-60C1 @ 0.70A TOTAL LOAD: 20 AMPS @ 120VAC

CIRCUITS: (1) 20 AMP REQ'D.

INSTALLATION ADDRESS:

CHASE BANK - ATM SOUTHINGTON QUEEN

885 QUEEN STREET SOUTHINGTON, CT 06489

LIENT:



520 WEST SUMMIT HILL DR, SUITE 702 - KNOXVILLE, TN 37902 Tel (865) 693-1105 - Fax (865) 693-1106 - Toli Free (866) 218-1976

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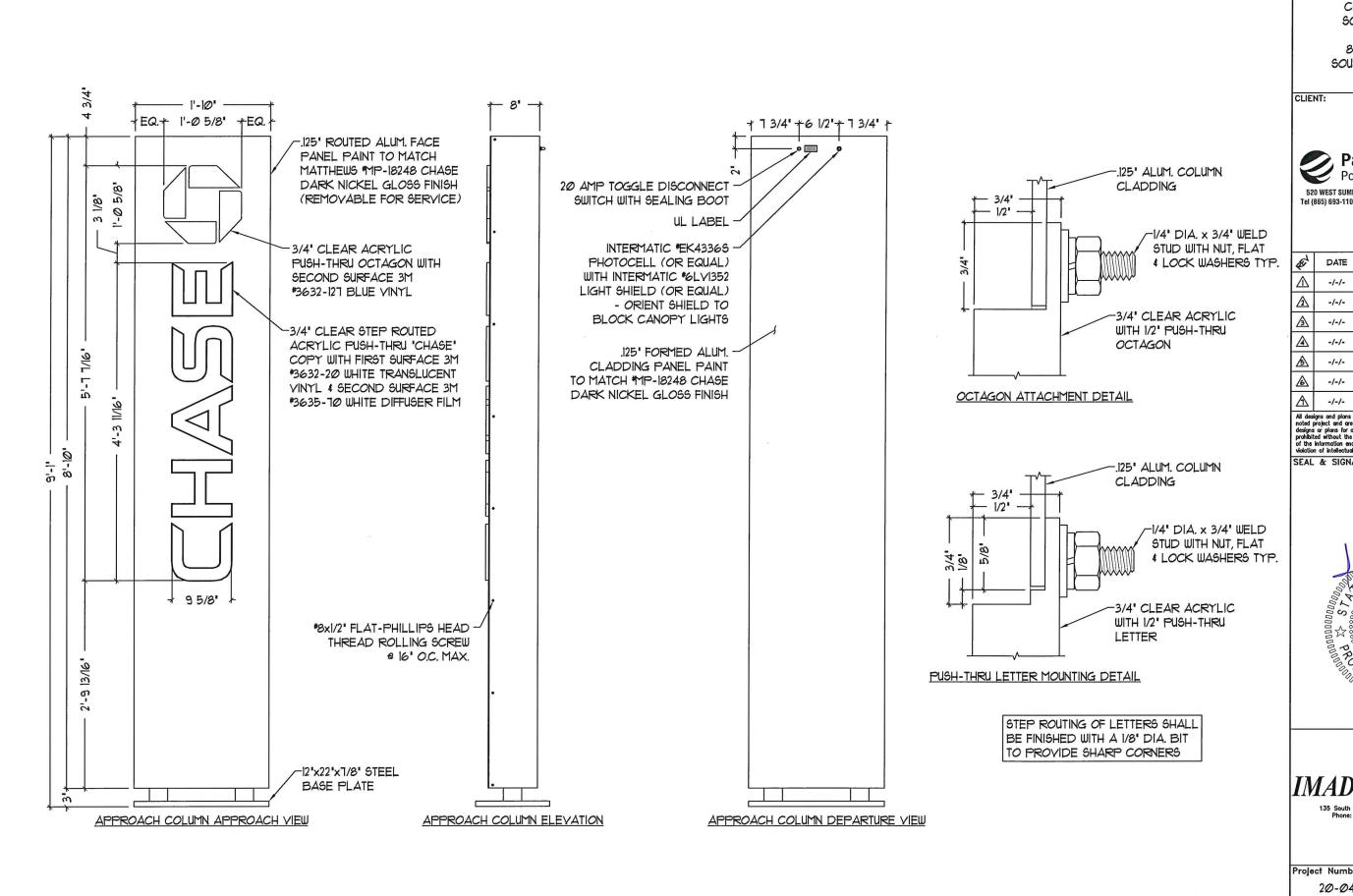
SEAL & SIGNATURE:



IMAD KASHIF, P.E.

135 South David Lane • Knoxville, Tennessee 37922 Phone: (865) 539-4001 • www.linkengr.com

Project I	Number:	Drawing Number:			
20-0439		B2582496			
SHT.	OF	DATE:	BY:		
- 1	10	6/30/20	TR		



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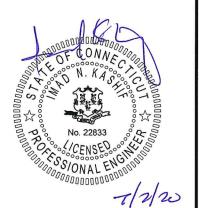


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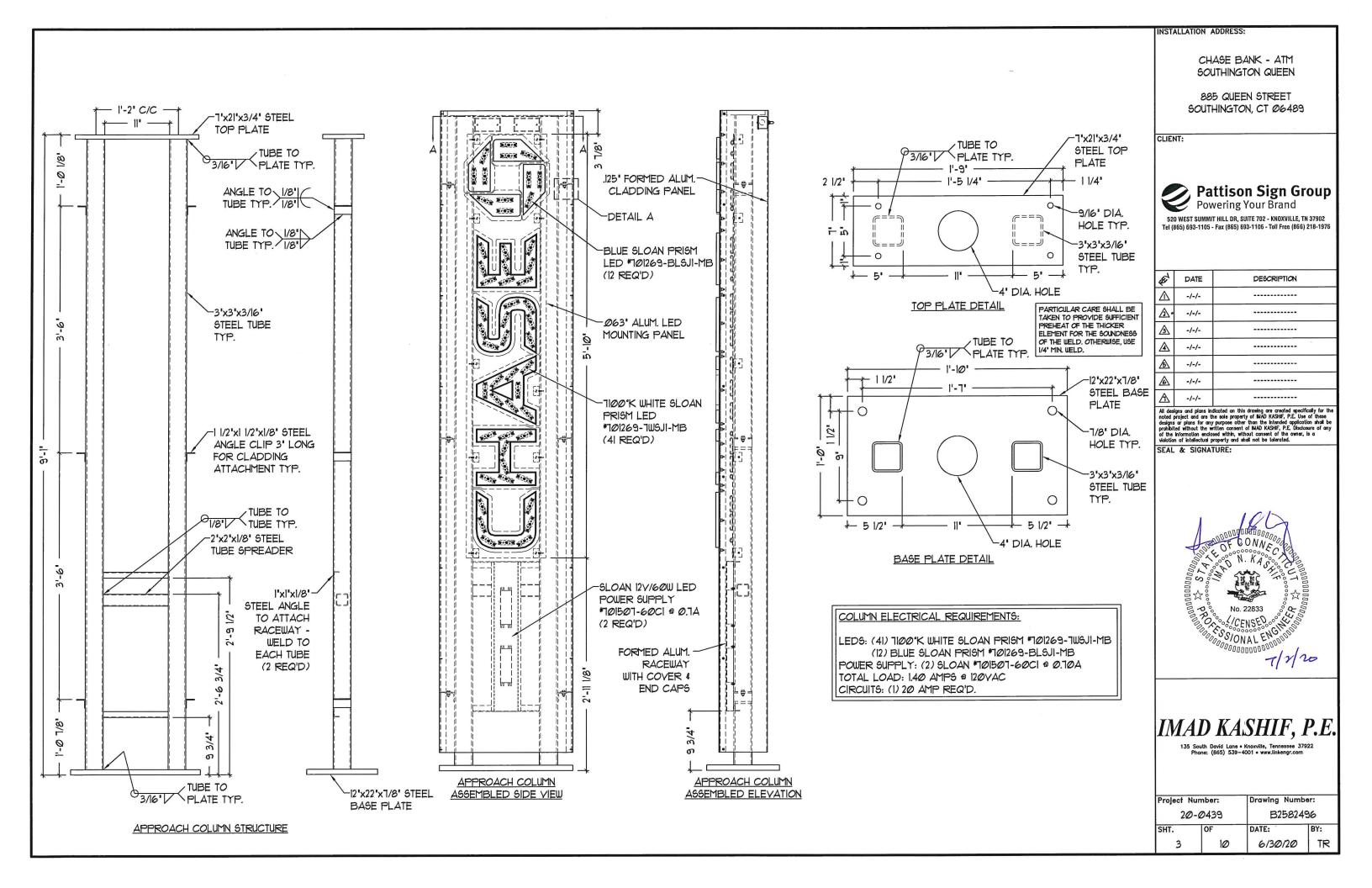
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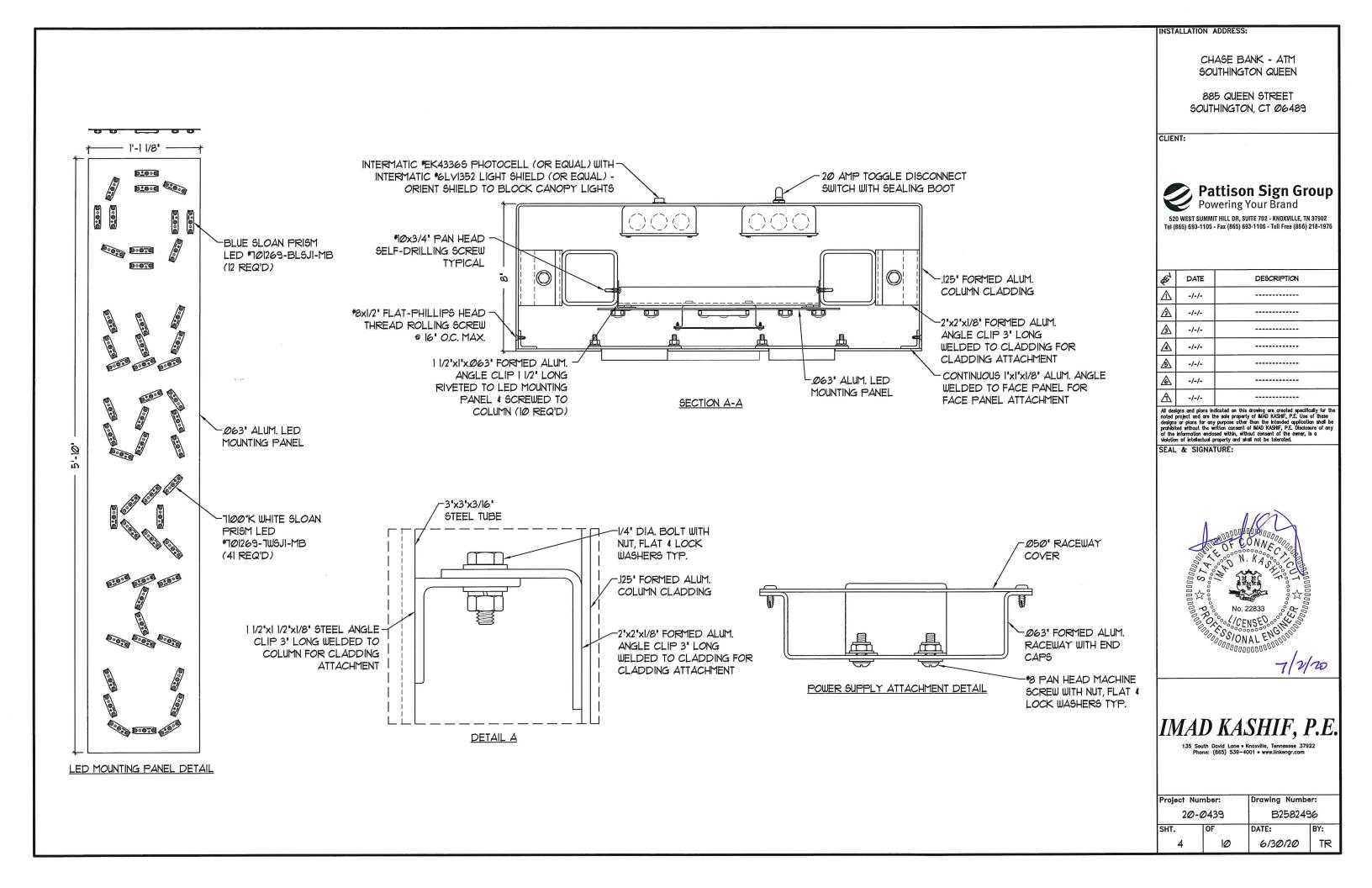


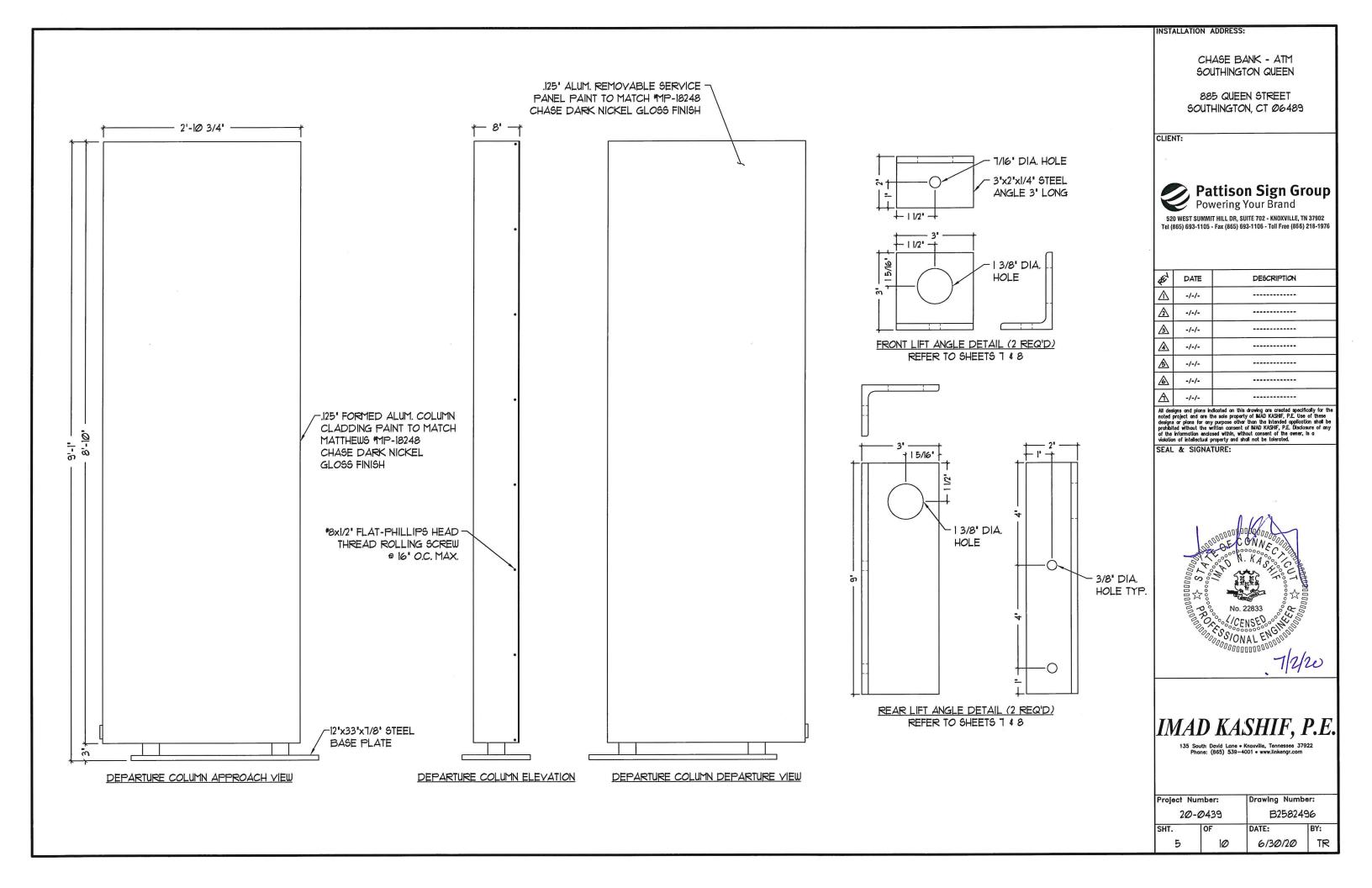
IMAD KASHIF, P.E.

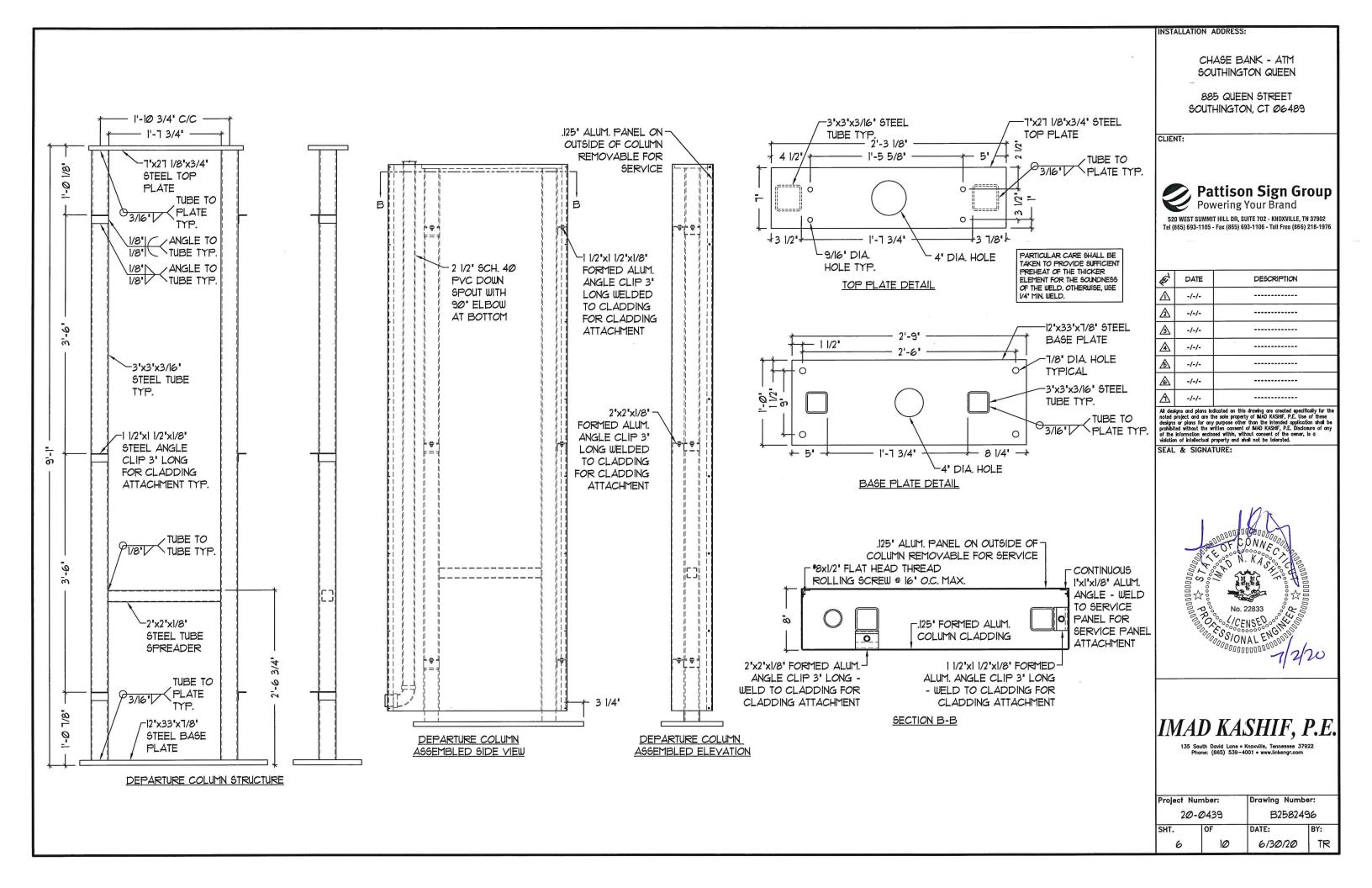
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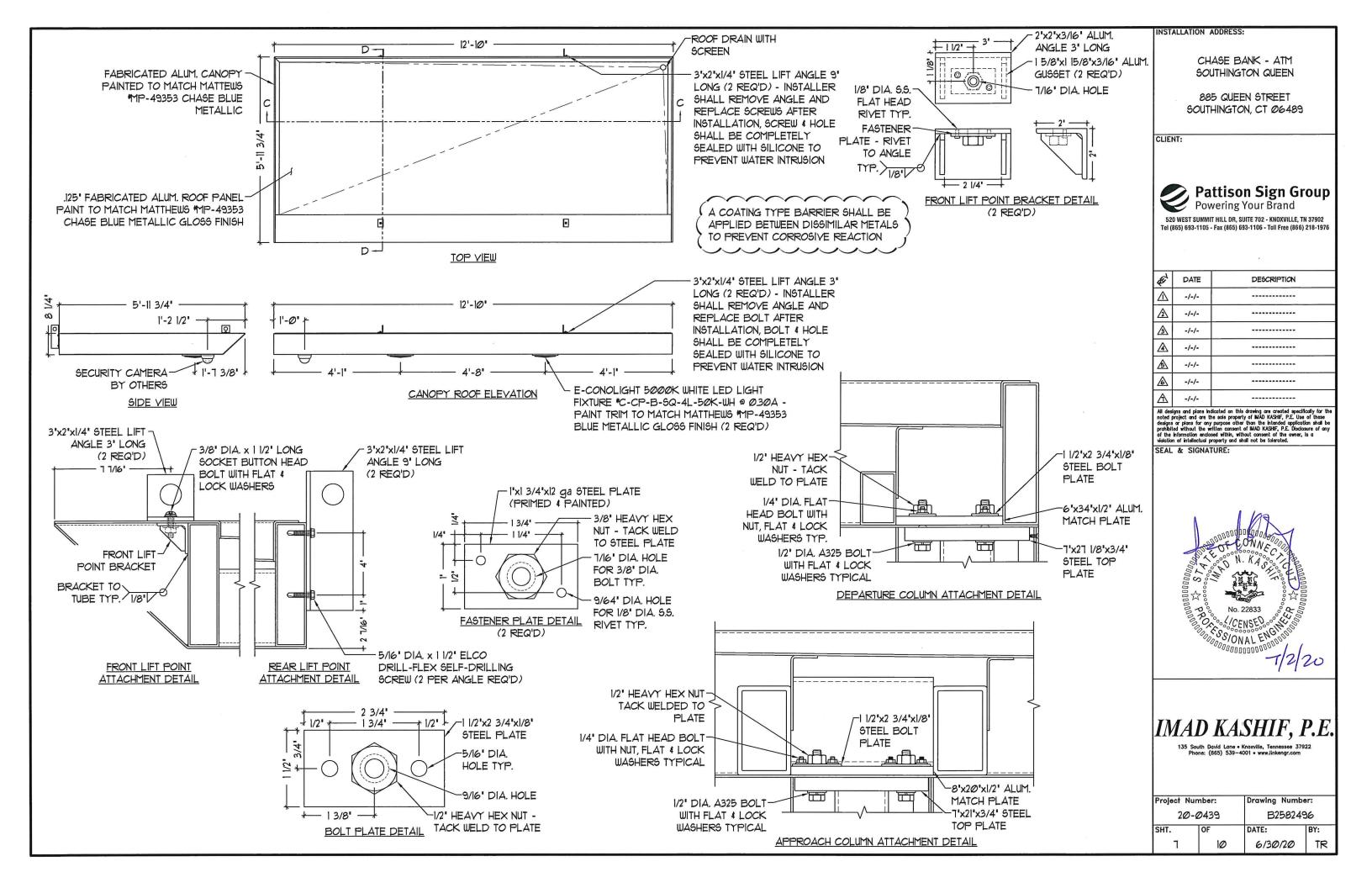
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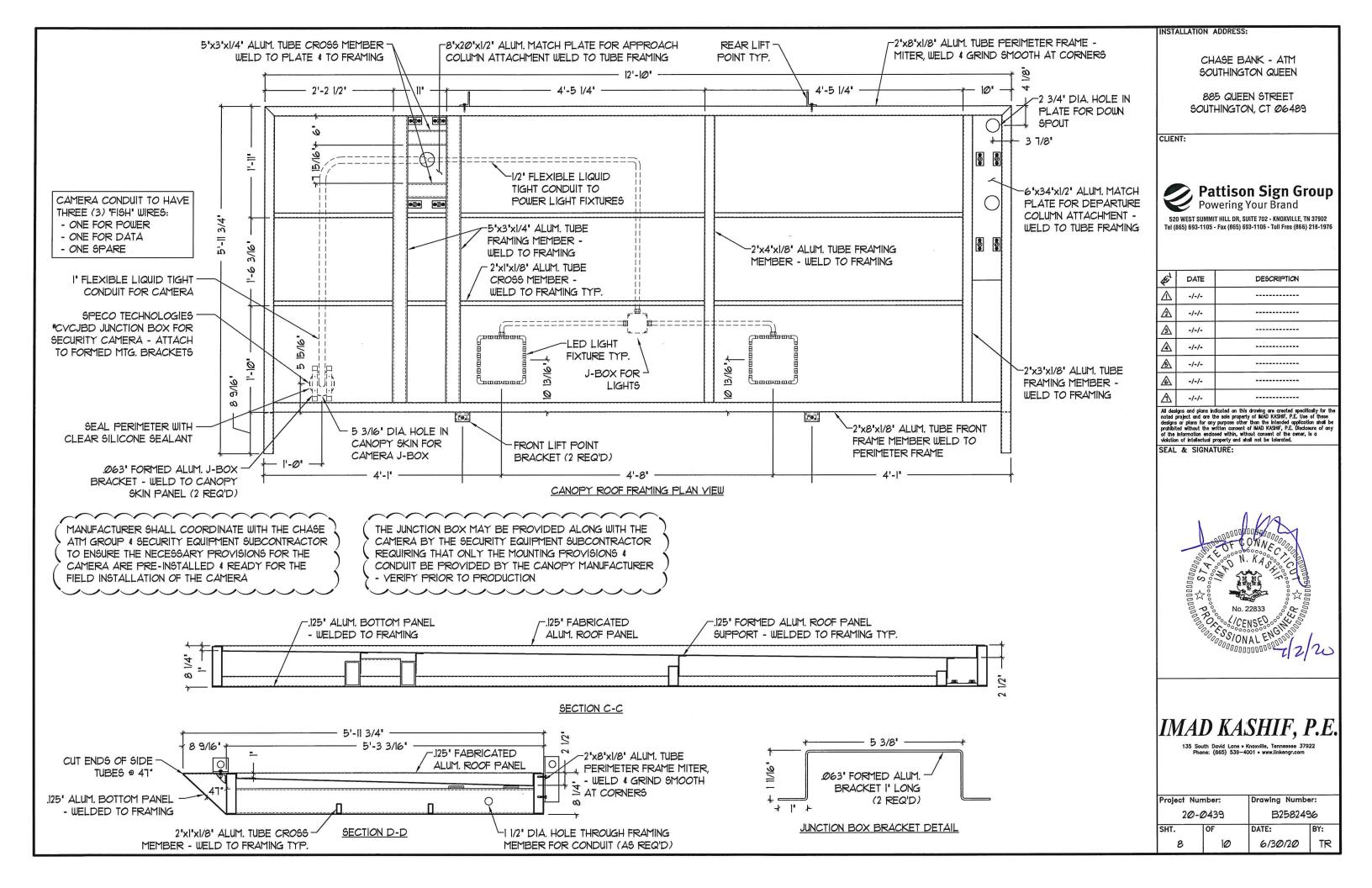


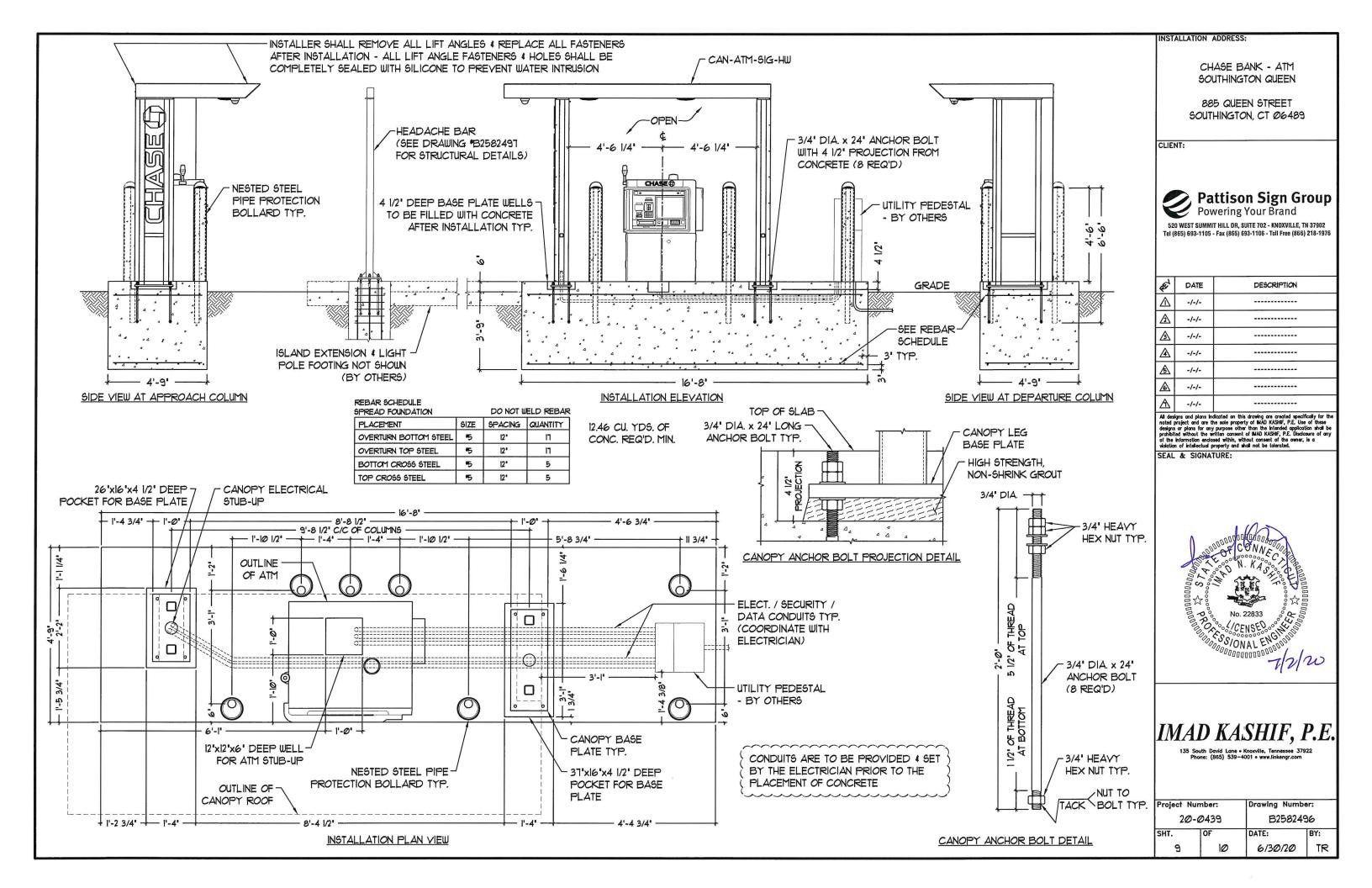












	PROJECT#	20-0439		OWNER-	CHASE-SC	OUTHINGTON QU	JEEN - ATM	
	June 30, 2020	25 5455		O.III	885 QUEEN			
	DRAWING#	B2582496				ON, CT 06489		
	WIND LOAD	30,14	PSF		30011111401	J. 1, J. 100403		
	WIND SPEED	130	MPH	CI IEVIT-	PATTISON S	SIGN GROUP-KN	OXVII I F	
	# COLUMNS		CSBC 2018/ IB			SUMMITHILL DR		
	DESIGNER	тос	CODC 2010/1D	02013	KNOXVILLE		, 50112702	
	DODIO! E.	100			IN YORV ILLE			
			SHAPE 1	CENTROID		TOTAL		Total
пвм 🔻	HEGHT	WDTH *	FACTOR	HEGHT	AREA T	FORCE	MOMENT	Momen
	1100111			======				Momen
CANOPY	0.688	12.667	1.000	0.346	8.708	0.263	0.091	0.091
COLUMNATM	3.253	1.333	1.000	1.626	4.337	0.393	1.157	
								1.157
COLUMN/ATM PEDESTAL	5.581 0.500	10.372 16.667	1.000	2.790 0.250	57.886 8.333	2.138	8.221 9.352	10.650
		10.007	1.000	0.230	0.555	2.309	9.332	11.782
OAH	10.021							
		Cantillaria v Ma	Dia 4-	Di and Down Life		2 420		
		Cantilever Mc	ment Due to	DL and DownLift		2.429		
	COLLBAIG	CALCULATIONS	(00000	D-DDC-O-OTI ICD-	T-71 IOO			
	COLUMN	CALCULATIONS	(COLES	P=PPE:O=OTHER			43/A II 4 DI E	
	O11351		0011551 ¥		DESIGN	חבסו ייסכדי	AVAILABLE	
mr.	∞LUMN	, COLUMN ,	COLUMN	bxx	MODULUS'		FLEXURAL	1000
mem .	WIDTH	DEPTH	WALL	COLUMN	COLUMN	MOMENT	STRENGTH	UNITY
COLLANGE	2.000	2.000	0.174	2.5	4.07		4.52	
COLUMNATM	3.000	3.000	0.174	2.5	1.97	1.495	4.53	0.440
						From Risa		
	ВС	LT CALCULATIO	NS					
						ALL COLUMN		
		BOLT	BOLTS/	TENSION	BOLT	ALLOW.	ALLOWABLE	
ПЕМ	MOMENT	SPACING	RATE	BOLT	DAM	STRESS	TENSION	
APPROACH COLUMNS	1.444	9.000	2.000	2.790	0.750	19.100	8.438	
DEPARTURE COLUMNS	1.358	9.000	2.000	1.695	0.750	19.100	8.438	
	From Risa			From Risa				
	PL/A	ATE CALCULATIO	ONS					

ΠEΜ	TENSION	MOMENT	MOMENT	PLATE	PLATE	PLATE	MINIMUM	
	BOLT	ARM	PLATE	WIDTH	DEPTH	THCK	THICK.	
APPROACH COLUMNS	2.790	3.281	9.156	4.563	22.000	0.875	0.668	
DEPARTURE COLUMNS	1.695	3.125	5.297	4.031	33.000	0.875	0.540	
ANCHOR BOLT PROJECTION	AN	ICHOR EMBEDME	MT	ANCHO	R BOLT MIN."L	_ENGTH		
4.500		7.402			12.000			
SPREAD FOUNDATION								
MOMENT AT GRADE					11.782			
TOTAL FORCE					2.389	kip		
WEIGHT OF SIGN					3.700	kip		
PEDESTAL WIDTH					16.667	ft		
PEDESTAL LENGTH					4.750	ft		
PEDESTAL THICKNESS					0.500	ft		
PEDESTAL WEIGHT					5.938	kip		
SLAB WIDTH					16.667	ft		
SLAB LENGTH					4.750	ft		
SLAB DEPTH					3.750	ft		
SLAB WEIGHT					44.532	kip		
TOTAL WEIGHT					54.170	kip		
OVERTURNING MOMENT					21.936			
FACTOR OF SAFETY					5.865			
e = OTMWT					0.405			
L/2 - e					1.970			
SOIL PRESSURE 2*WT/(3*(L/2	2-e)*WIDTH)				1100			
CONCRETE					12.462	yd ³		
EXCAVATION					10.996	yd ³		
ENCAVATION					6.174	in		
MIN., THICKNESS W/O REBAR	₹							
					0.114	582 [

BOTTOM STEEL AREA REQ'D PER FT OF WIDTH		0.029	
TOP STEEL AREA REQ'D PER FT OF WIDTH		0.034	
LONG BOTTOM STEEL	REBAR SIZE	5.000	
	WEIGHT PER FT	1.043	
	SPACING	12.000	in
	AREA PER BAR	0.310	in
BOTTOM STEEL AREA REQ'D PER FT OF WIDTH		0.029	
	AREA PER FT	0.316	
	EST NO. REQ'D	16.667	
	NUMBER REQ'D	17.000	
	LENGTH	4.250	ft
	WEIGHT	75.357	
	EDGE	4.002	
LONG TOP STEEL	REBAR SIZE	5.000	
	WEIGHT PER FT	1.043	
	SPACING	12.000	in
	AREA PER BAR	0.310	in
TOP STEEL AREA REQ'D PER FT OF WIDTH		0.034	
	AREA PER FT	0.316	
	EST NO. REQ'D	16.667	
	NUMBER REQ'D	17.000	
	LENGTH	4.250	ft
	WEGHT	75.357	
	EDGE	4.002	
CROSS STEEL	REBARSIZE	5.000	
	WEIGHT PER FT	1.043	
	SPACING	12.000	in
	LENGTH	16.167	ft
	EST NO. REQ'D	4.750	
	NUMBER REQ'D	5.000	
	EDGE	4.500	in
	WEGHT	168.622	
	TOTAL WEIGHT	319.335	
FOUNDATION WIDTH		16.667	ft
FOUNDATION LENGTH		4.750	ft

General Notes:

- Design is based on a 130 mph, 3 second gust wind design per the 2018 Connecticut State Building Code / IBC 2015. Category II, Exposure C. 30 psf Ground Snow Load. Seismic Design Category B.
- Spread foundation is based on a presumptive safe vertical soil bearing pressure minimum of 2000 psf.
- 3. A soil report was not provided. Foundation analysis assumes Soil Classification 4. Allowable bearing pressure should be verified prior to placement of concrete. In the event that the stated requirements are not met and conditions appear deleterious, cease and secure excavation and immediately contact Pattison Sign Group.
- 4. Foundation shall not be placed at the top of, or on the side of a slope exceeding 3:1, or adjacent to a fill slope unless re-evaluated by a competent Professional Engineer. Do not place foundation in fill.
- Concrete shall be mixed to attain a minimum 28 day compressive strength of 3000 psi.
- Steel reinforcing bars shall conform to ASTM A615, Grade 60 with deformations in accordance with ASTM A305. Welding of reinforcing bars is prohibited.
- All voids between column base plate and foundation surface shall be completely filled with high-strength, non-shrink grout.
- Anchor bolts shall meet ASTM FIB54 Grade 36. Exposed surfaces shall be galvanized or coated to prevent corrosion.
- 9. All support members shall be free from defects. Steel tube shall meet ASTM A500 Grade B with a minimum yield strength of 46000 psi. Steel angle, channel and plate shall meet ASTM A36. Aluminum shapes shall be 6061-T6 alloy. Aluminum plates shall be 5052-H34 alloy. Aluminum sheet shall be 3003-HI4 alloy.
- 10. Steel welds shall be made with E70xx low hydrogen electrodes. Aluminum welds shall be made with 5356 filler. All welds shall be made by persons qualified in accordance with AWS standards within the past two years.
- II. All structural bolts shall conform to ASTM A325, and be zinc coated unless noted otherwise. When used with structural bolts, heavy hex nuts shall conform to ASTM A563, and washers shall conform to ASTM F436. Pretension all high strength bolts using the Turn-of-Nut method unless noted otherwise.
- The scope of this engineer does not include onsite observations.
- 13. IMAD KASHIF, P.E. will not be responsible for the safety on this job site before, during or after installation of this structure. It is the responsibility of the owners, contractors and installers to ensure that the installation and erection of this structure is performed using methods that are in full compliance with OSHA regulations.
- 14. Any deviation from this design or from any part of this drawing, including the General Notes, without prior written consent from IMAD KASHIF, P.E. voids this drawing in its entiretu.
- 15. The structure designed on this drawing is intended to be installed at the address shown and should not be used at any other location.

INSTALLATION ADDRESS:

CHASE BANK - ATM SOUTHINGTON QUEEN

885 QUEEN STREET SOUTHINGTON, CT 06489

CLIENT:



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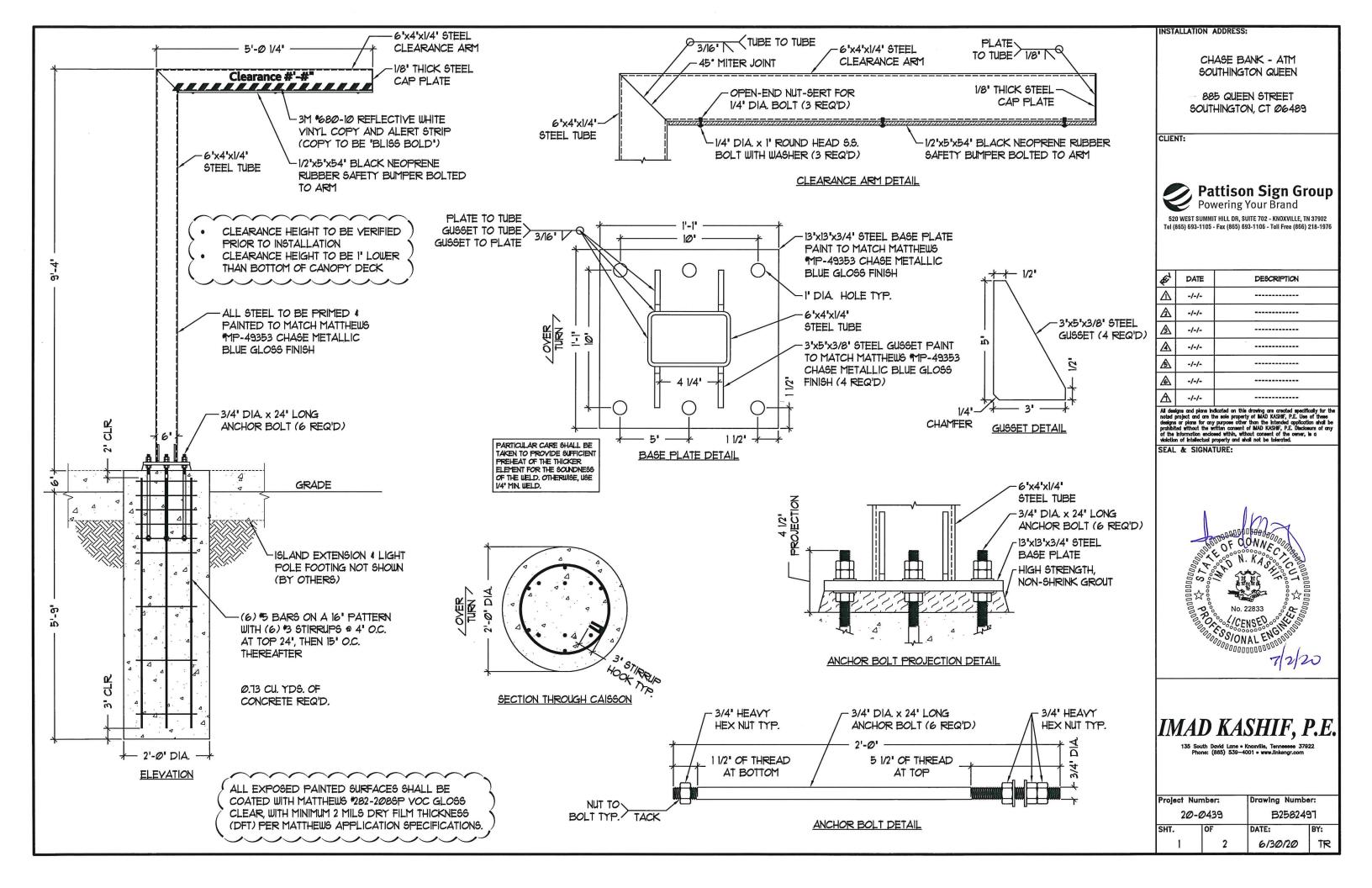
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IMAD KASHIF, P.E.

135 South David Lane • Knoxville, Tennessee 37

Project N	umber:	Drawing Number:			
20-0439		B2582496			
SHT.	OF	DATE:	BY:		
10	10	6/30/20	TR		



		PROJECT#	20-0439		OWNER:	CHASEBAN	K - SOUTHING	TON QUEEN - A TM		
		June 30, 2020	20 0 100		OTHILL.	885 QUEEN S		OTT GOLLIT 7TIII		
		DRAWING#	B2582497				ON, CT 06489			
		WIND LOAD	29.45	PSF		000111111011	514, 61 00405			
		WIND SPEED	130	MPH	CLIENT:	PATTISON S	ICN CROUD			
								TE 404		
		# COLUMNS	1 '	CSBC 2018/ IB	C 2015		R BLUFF RD S	IE 101		
		DESGINER	TCC			KNOXVILLE,	IN			
					West and the second second					
				SHAPE '	CENTROID		TOTAL		CANTILEVER	IMPACT
	ITEM	' HEIGHT '	WDTH	FACTOR	HEIGHT	AREA `	FORCE	MOMENT	MOMENT	MOMEN
	=======			=======	=======					
	CL. BAR	0.542	5.021	1.000	0.271	2.720	0.080	0.022		
	COLUMN	8.792	0.500	1.000	4.396	4.396	0.210	1.473	0.178	6.750
	OAH	9.333								
		COLUMN CA	LCULATIONS	(CODES	P=PIPE;O=OTHER	T=TUBE)				
						DESIGN		AVAILABLE		
		COLUMN	COLUMN	COLUMN	lxx	MODULUS"	REQUIRED	FLEXURAL		
	ITEM	WDTH	DEPTH	WALL	COLUMN	COLUMN	MOMENT	STRENGTH	UNITY	
٦,	=======	. ======	=======	=======	======	=======		=======	=======	
	COLUMN	6.000	4.000	0.233	11.1	6.45	6.928	14.80	0.468	
	COLOIVIIV	0.000	4.000	0.233		0.40	0.525	14.00	0.400	
		POL	T CALCULATION	ONIC	TO BE STATE OF THE					
			1 CALCULA 110							
					7711010111					
			BOLT	BOLTS/	TENSION	BOLT	ALLOW.	ALLOWABLE		
	ITEM	MOMENT	SPA CING	PLATE	BOLT	DIAM.	STRESS	TENSION		
	BASE PL.	6.928	10.000	6.000	2.771	0.750	19.100	8.438		
		PLA1	ECALCULATI	IONS						
			*****	•						
	ITEM	TENSION	MOMENT	MOMENT	PLATE	PLATE	PLATE	MINIMUM		
		BOLT	ARM	PLATE	WDTH	DEPTH	THICK.	THICK.		
	BASE PL.	2.771	3.000	24.941	13.000	13.000	0.750	0.653		
,	A NCHOR BOLT PROJ	IECTION AND	HOR EMBEDM	ENT	ANCHO	R BOLT MIN."L	.ENGTH			
Ť	4.500		7.351			12,000				
	1.000		7.001			12.000				
	CAISSON									
	MOMENT					6.928	FT-KIP			
						0.750	KIP			
	FORCE	7 2 2 9 TA DI E 4000 0				0.750	NP			
		7.3.2 & TABLE 1806.2	0110.00							
		S #4 SW, SP, SM, SC,								
		PRESSURE - PSF/FT OF	DEPTH			150.0	PSF/FT			
	S1					575.0				
I	DEPTH					5.750	FT.			
I	DIAMETER					2.000	FT.			
						9.237	FT.			
						1.526	FT.			
(CALCULATED DEPTH	1				4.757	FT.			
ı	MINIMUM THICKNESS	WITHOUT REINFORCE	MENT			21.709	IN.			
	A CTUAL DIA METER					24.000	IN.			
						1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2				

General Notes:

1. Design is based on a 130 mph, 3 second gust wind design per 2018 Connecticut State Building Code / IBC 2015. Category II, Exposure C. 30 pef Ground Snow Load. Seismic Design Category B.

2. Calsson foundation is based on a presumptive safe lateral soil bearing pressure minimum of 150 per foot of depth. Isolated lateral bearing footings subject to short-term lateral loads and not adversely affected by a 1/2' motion at grade are permitted to be designed using twice the tabulated value of the corresponding soil class

A soil report was not provided. Foundation analysis assumes Soil Classification 4. Allowable bearing pressure should be verified prior to placement of concrete. In the event that the stated requirements are not met and conditions appear deleterious, cease and secure excavation and immediately contact Pattison Sign Group.

4. Foundation shall not be placed at the top of, or on the side of a slope exceeding 3:1, or adjacent to a fill slope unless re-evaluated by a competent Professional Engineer. Do not place foundation in fill.

5. Concrete shall be mixed to attain a minimum 28 day compressive strength of 3000 psi.

6. Steel reinforcing bars shall conform to ASTM A615, Grade 60 with deformations in accordance with ASTM A305. Welding of reinforcing bars is prohibited.

All voids between column base plate and foundation surface shall be completely filled with high-strength, non-shrink grout.
 Anchor bolts shall meet ASTM FIB54 Grade 36. Exposed

surfaces shall be galvanized or coated to prevent corrosion.

9. All support members shall be free from defects. Steel tube shall meet ASTM A500 Grade B with a minimum yield strength of

46000 psi. Steel angle, channel and plate shall meet ASTM A36.

10. Steel welds shall be made with ET0xx low hydrogen electrodes. All welds shall be made by persons qualified in accordance with All Standards within the past two years.

All structural bolts shall conform to ASTM A325, and be zinc

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12. The scope of this engineer does not include onsite observations.

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